



PBPower

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Calculation Cover Sheet

project: Cosumnes Power Plant **jobno.:** 13578 **discipline:** Civil
subject: Detention Basin Routing Analysis **calculationno.:** 13578-C-1
originator: L. Gasparetti **date:** 1/11/02 **fileno.:**
checker: **date:** **Sheet 1 of 7 Sheets**

Purpose of Calculation

Provide reservoir routing analysis for detention basin.

Summary of Results and Conclusions Starts in Sheet No. _

Sources of Design Criteria

Sources of Formula and References:

FR-1 Rainfall Depth Duration Frequency for Eagles Nest (Department of Water Resources)
FR-2 Calculation No. 13561-C-1

Record of Issues

No.	Description	By	Date	Checked	Date	Approved	Date	Date Filmed
Preliminary Calculation		X	Committed Preliminary Design Calculation					
Superseded Calculation			Final Calculation					



Calculation Sheet

project:	SMUD-CosumnesPowerPlant	jobno.:	13578	discipline:	Civil
subject:	DetentionBasinRoutingAnalysis	calculationno.:	13578-C-1		
originator:	L.Gasparetti	date:	1/11/02	fileno.:	
checker:		date:		Sheet 2 of 7 Sheets	

A. PURPOSE

Seecoversheet.

B. METHODOLOGY

1. Definemaximuminflowhydrographandfindmaximumdifferencebetweenareasundertrapezoidal inflowhydrographanddesiredbasindischargerate.
2. TheRationalMethoddeterminesthepeakrunoff:
 $Q = CiA$, where
 Q = peakrunoff(cfs)
 C = runoffcoefficient
 i = rainfallintensity(in/hr)(basedontimeofconcentration, T_c)
 A = contributingarea(acres)

C. DESIGNCRITERIA

1. Use10-yearfrequency.
2. Use"C"fordevelopedsitefromFR-2: $C = 0.70$
3. Use"A"fordevelopedsitefromFR-2: $A_{DEV} = 25.2ac$
4. Determine" i "fromFR-1: $i = \text{maxrainfall(in)}/T_c(\text{hr})$

D. ASSUMPTIONS

1. Forthepurposeofthispreliminarycalculation,assumethattheintial $T_c = 10$. Therefore,therunoff fromalltributaryareasshouldpeakat10minutes,which isoftenconsideredtheshortestpractical T_c thatproducesthehighestaverageintensity.

E. INFLOWHYDROGRAPH

$T_c(\text{min})$	$i(\text{in/hr})$	CxA	$Q(\text{cfs})$
10	1.74	17.64	30.69
15	1.36	17.64	23.99
30	1.02	17.64	17.99
40	0.87	17.64	15.34
50	0.82	17.64	14.46
60	0.76	17.64	13.41
2hr	0.53	17.64	9.35
3hr	0.45	17.64	7.94
6hr	0.29	17.64	5.12
12hr	0.20	17.64	3.47
24hr	0.11	17.64	1.94

F. OUTFLOWHYDROGRAPH

Outflowisrestrictedtotheamountofrunofffromtheundevelopedsite: $Q_{UDEV} = 0.83\text{cfs}$

A00 H Eagles Nest SC

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Calc. No. 13578-C-1
Source: FR-1
Date 1/11/02

Rainfall Depth Duration Frequency for Eagles Nest

DWR # A00 2160 34

Sacramento County

Latitude 38.485°

Analysis By DWR DLA

Longitude -121.260°

Data From : DWR, Sac Co Sta # 269

TB 340-B-3

Elevation 100 Feet

	Maximum Rainfall For Indicated Number Of Consecutive Days												
	5 Min	10 Min	15 Min	30 Min	1 Hr	2 Hr	3 Hr	6 Hr	12 Hr	1 Day	2 Day	3 Day	F Yr
1976			0.20	0.34	0.44	0.58	0.83	0.95	0.98	0.98			6.08
1977			0.15	0.24	0.30	0.36	0.39	0.55	0.71	0.82			5.89
1978					0.52	0.97	1.04	1.19	1.32	1.87			21.71
1979			0.13	0.17	0.27	0.43	0.58	0.88	1.03	1.20			15.72
1980			0.40	0.60	0.80	1.13	1.23	1.38	1.51	1.54			21.86
1981	0.05	0.09	0.14	0.18	0.29	0.47	0.61	0.93	1.25	1.64			12.43
1982			0.10	0.15	0.30	0.38	0.51	0.84	1.00	1.47			28.04
1983	0.15	0.20	0.20	0.35	0.65	0.90	1.10	1.40	1.78	2.73			35.89
1984	0.10	0.20	0.30	0.45	0.70	1.04	1.25	1.39	1.95	2.63			19.04
1985	0.06	0.12	0.18	0.36	0.56	0.74	0.91	1.01	1.31	1.32			10.99
1986	0.12	0.20	0.24	0.35	0.51	0.79	1.02	1.61	1.97	2.20	4.02	5.16	16.97
1987	0.04	0.08	0.12	0.24	0.28	0.39	0.47	0.71	1.18	1.73	1.77	2.01	12.01
1988	0.04	0.12	0.16	0.31	0.47	0.55	0.83	1.14	1.85	2.28	2.40	2.40	13.23
1989	0.08	0.16	0.16	0.24	0.39	0.47	0.59	0.79	1.02	1.14	1.50	1.73	14.69
1990	0.16	0.20	0.24	0.31	0.47	0.71	0.87	1.06	1.30	1.30	1.57	1.65	15.00
1991	0.20	0.24	0.24	0.24	0.43	0.63	0.91	1.30	1.57	1.57	1.85	2.68	14.91
1992	0.12	0.20	0.24	0.35	0.59	0.87	1.06	1.46	1.61	1.69	2.32	2.99	15.36
1993	0.16	0.20	0.28	0.28	0.47	0.51	0.63	0.79	1.38	1.81	2.28	0.20	16.45
1994	0.16	0.20	0.24	0.35	0.47	0.59	0.91	1.14	1.18	1.18	1.42	2.42	10.04
1995	0.28	0.47	0.59	0.87	1.02	1.26	1.65	1.97	2.24	2.60	3.03	3.58	26.06
1996	0.16	0.28	0.35	0.63	0.75	1.06	1.14	1.38	5.10	1.97	3.03	3.31	20.86
1997	0.24	0.31	0.31	0.51	0.91	1.18	1.42	1.57	1.85	2.44	2.87	3.31	21.50
1998	0.12	0.16	0.16	0.31	0.43	0.71	0.94	1.46	2.13	2.87	3.50	3.82	31.11
1999	0.08	0.12	0.16	0.20	0.43	0.75	0.98	1.22	1.22	1.38	1.54	2.52	15.36
2000													
Average	.13	.20	.23	.35	.52	.73	.91	1.17	1.60	1.77	2.36	2.70	17.55
Stdev	.07	.09	.11	.17	.20	.27	.31	.34	.85	.59	.82	1.17	7.36
Rec Max	.28	.47	.59	.87	1.02	1.26	1.65	1.97	5.10	2.87	4.02	5.16	35.89
Rec Min	.04	.08	.10	.15	.27	.36	.39	.55	.71	.82	1.42	.20	5.89
Z	3.33	3.93	4.45	4.24	2.75	2.08	2.30	1.94	6.21	1.78	.52	.39	3.37
Yrs Rec	18	18	23	23	24	24	24	24	24	24	14	14	24
CV	.523	.463	.477	.484	.385	.373	.339	.287	.529	.332	.347	.435	.419
Reg CV	.352	.352	.352	.352	.352	.352	.352	.352	.352	.352	1.352	2.352	.310
alc Skew	.6	1.6	1.8	1.6	1.0	.4	.4	.3	3.2	.4	.6	.0	.8
Reg Skew	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	0.4
FIC	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
RP 2	.12	.18	.22	.33	.49	.68	.85	1.10	1.50	1.65	1.79	1.56	17.19
RP 5	.16	.25	.29	.44	.65	.92	1.15	1.48	2.02	2.23	4.75	7.43	21.99
RP 10	.19	.29	.34	.51	.76	1.07	1.34	1.72	2.36	2.60	6.65	11.21	24.72
RP 25	.22	.34	.40	.60	.90	1.26	1.57	2.02	2.77	3.05	8.97	15.81	27.78
RP 50	.25	.38	.44	.67	.99	1.39	1.74	2.24	3.06	3.37	10.63	19.11	29.85
RP 100	.27	.41	.48	.73	1.08	1.52	1.90	2.44	3.34	3.68	12.23	22.29	31.78
RP 200	.29	.45	.52	.79	1.17	1.64	2.06	2.65	3.62	3.99	13.79	25.39	33.59
RP 500	.32	.49	.57	.87	1.29	1.81	2.26	2.91	3.98	4.38	15.84	29.46	36.00
RP 1000	.34	.52	.61	.92	1.37	1.93	2.41	3.10	4.24	4.67	17.30	32.36	37.49
RP 10000	.41	.63	.73	1.11	1.65	2.31	2.90	3.72	5.09	5.61	22.13	41.96	42.56

JDG

$i = \frac{\text{max rainfall}}{\text{hr}}$

Page 1

8/1/01

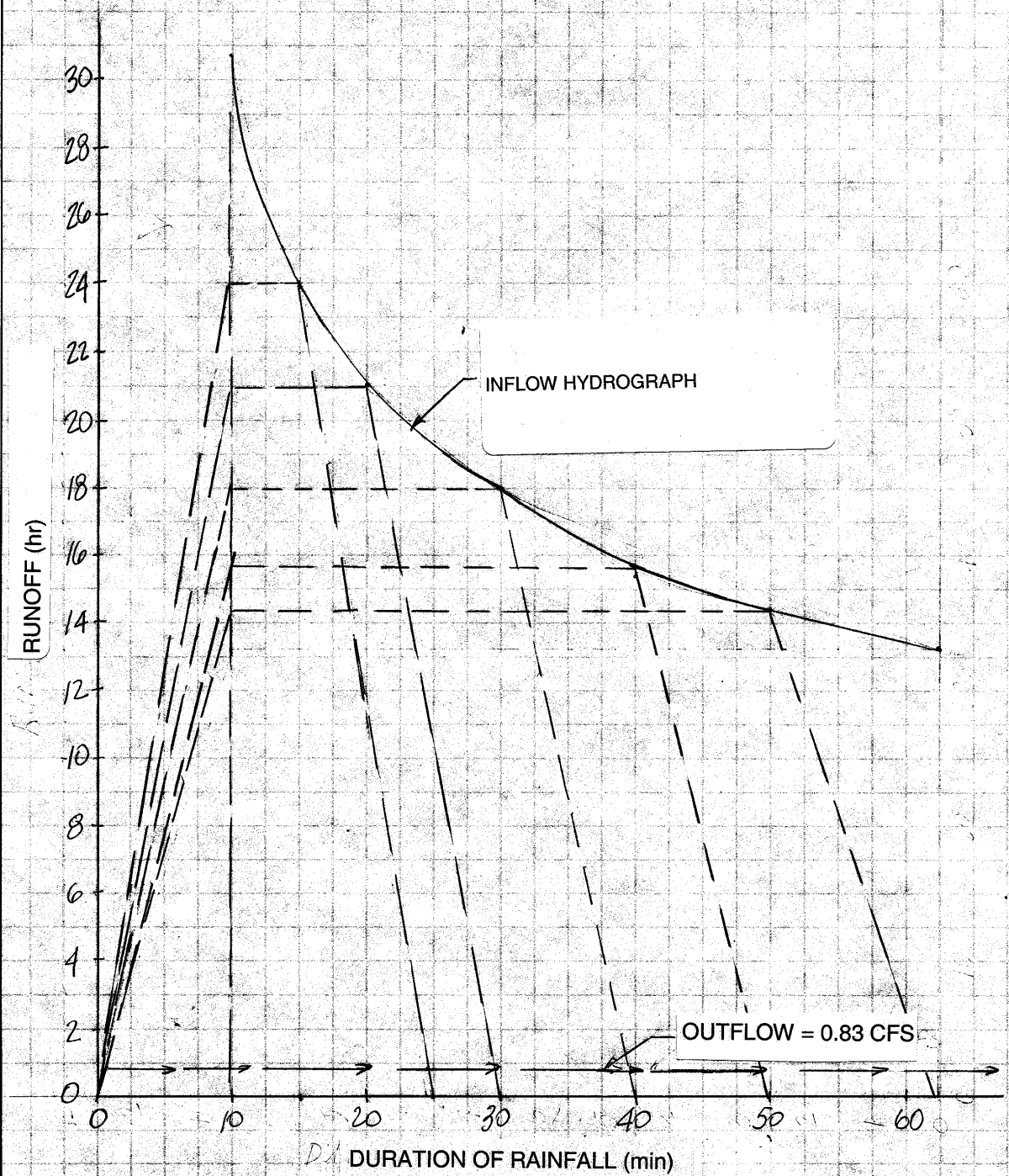


PARSONS BRINCKERHOFF COMPUTATION SHEET

Subject

SMUD - Consumnes Power Plant
Detention Basin Routing Analysis

Sheet 4 of 7
Calc. No. 13578-C-1
Originator L. Gasparetti
Date 1/11/02
Checked by
Date

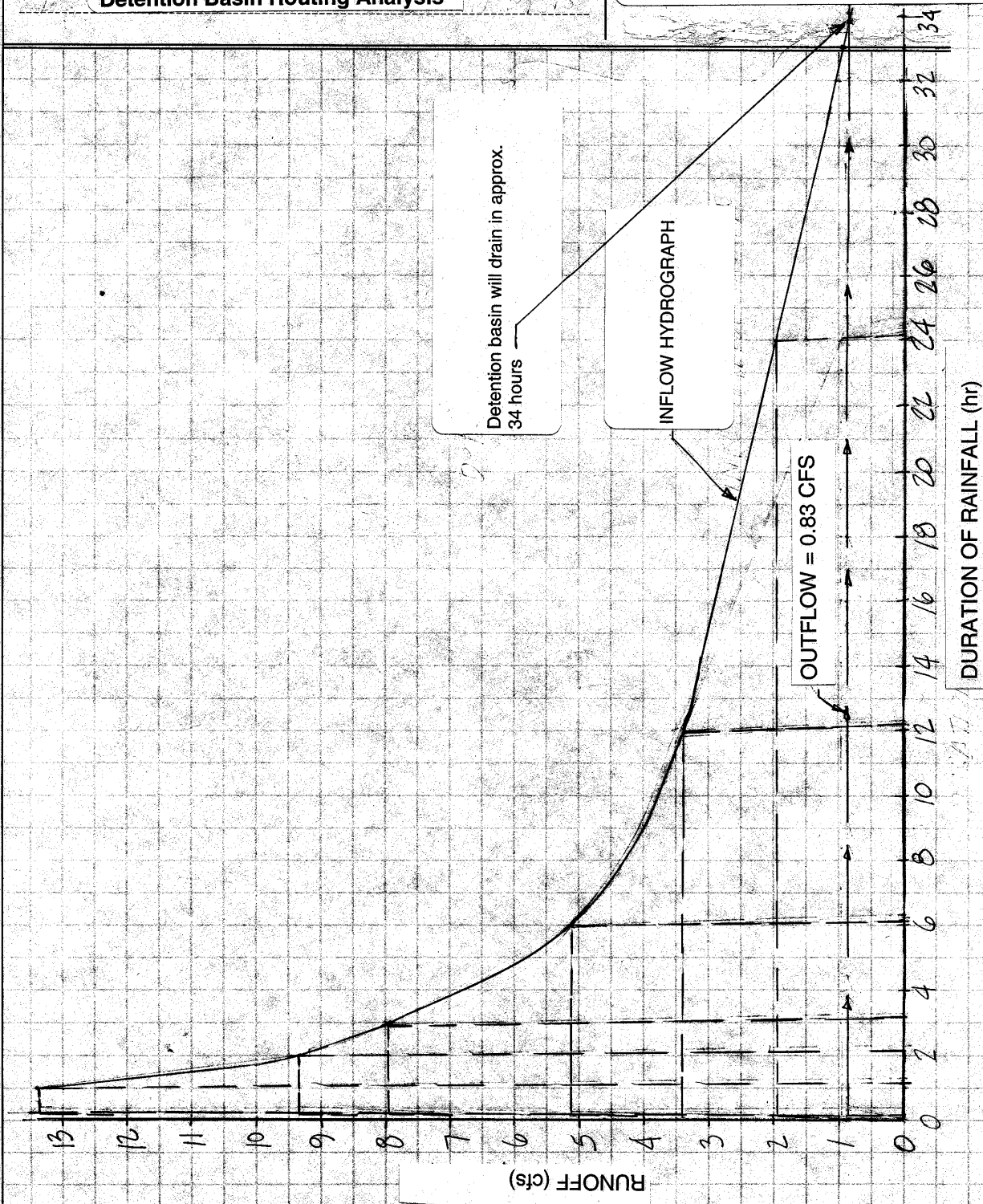




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G. STORAGE VOLUMES

$$V_{TC} = A_{\text{TRAPEZOID}}(60 \text{ sec/min})(Q_{\text{in}} - Q_{\text{out}})$$
$$V_{15} = ((5+24)/2)(60)(24-0.83) = 20,200 \text{ ft}^3$$
$$V_{30} = ((20+39)/2)(60)(18-0.83) = 31,700 \text{ ft}^3$$
$$V_{60} = ((50+63)/2)(60)(13.41-0.83) = 42,600 \text{ ft}^3$$
$$V_{3\text{hr}} = ((170+183)/2)(60)(7.94-0.83) = 75,300 \text{ ft}^3$$
$$V_{6\text{hr}} = ((350+363)/2)(60)(5.12-0.83) = 91,500 \text{ ft}^3$$
$$V_{12\text{hr}} = ((710+720)/2)(60)(3.47-0.83) = 113,300 \text{ ft}^3$$
$$V_{24\text{hr}} = ((1430+1440)/2)(60)(1.94-0.83) = 95,600 \text{ ft}^3$$



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